TUXEDO RESERVE PERFORMANCE STANDARDS FOR

Stormwater Management

Grading and Steep Slope Protection

Road Standards

Sanitary Sewer

Water Supply

Soil Erosion and Sediment Control

Tree Surveys

Water Quality Testing

Environmental Compliance

Rock Blasting and Stabilization

DOCUMENT INTENT

These Performance Standards, which consist of detailed engineering and construction specifications and requirements, are a component of the Design Standards which have been incorporated into and made part of the Special Permit for Tuxedo Reserve. They have been designed to address conditions specific to the Tuxedo Reserve site. In the event of any conflict between a Performance Standard and an otherwise applicable general Town standard, the Performance Standard shall apply unless such standard is otherwise expressly prohibited by the provisions for Design Standards in the Planned Integrated Development Law under which the Project is grandfathered. In the instances where a Performance Standard is not identified, the relevant Town of Tuxedo or New York State standard, whichever is more stringent, shall apply. The Planning Board is hereby authorized during its site plan and/or subdivision review to grant a Waiver or Waivers to the Performance Standards on a case by case basis provided the Planning Board finds justification for the grant of such Waiver on the following specific grounds:

- a. the Waiver is the result of a site or design condition that renders it impracticable to achieve strict compliance with the Performance Standards;
- b. grant of the Waiver will not cause an adverse environmental impact;
- c. grant of the Waiver will not create any nuisance or unsafe condition, and
- d. grant of the Waiver is consistent with the design intent of the Preliminary Plan and Special Permit.

A. STORMWATER MANAGEMENT

The stormwater management system shall be designed in accordance with the current requirements of the <u>NYS Stormwater Management Design Manual</u>.

Design shall meet the NYSDEC sizing requirements for water quality volume, channel protection volume, overbank flood control and extreme flood control. Requirements for channel protection overbank and extreme flood control may be waived in certain instances where the conditions specified in the <u>NYS Stormwater Management Design Manual</u> are met.

Where required, stormwater management systems shall be designed to provide for a zero net increase in the peak rate of stormwater runoff from the developed project to Offsite areas. In addition to the water quantity structural controls to be provided, grassed swales, water quality basins or other means will be developed to provide for water quality control measures. These water quality control best management practices will be provided in accordance with current NYSDEC requirements. Appropriate hydrologic and hydraulic calculations will be provided to demonstrate project impact to receiving channels or drainage systems immediately downstream.

1. Storm Drain System

- a. Drainage Collection System
 - 1) The drainage collection system should be designed to convey the 25-year storm event.
 - 2) The Rational Method Q=CIA shall be used to size storm water conveyance pipes where the drainage area is less than half a square mile or 320 acres.

Q =The peak runoff rate in cubic feet per second (CFS) C = The composite runoff coefficient based on the surface conditions:

<u>Condition</u>	<u>C</u>	

<u>U</u>
0.65
0.59
0.99

- I = The average rainfall intensity in inches per hour, taken from the intensityduration- frequency curve for Orange County, NY.
- Tc = The time of concentration in minutes, and the minimum Tc shall be 6 minutes.
- A = The size of the drainage area in acres.
- 3) USDA Soil Conservation Service Technical Release TR-20 methodology shall be used to size stormwater conveyance pipes where the drainage area is greater than half a square mile or 320 acres.
- 4) The stormwater management report shall analyze the 25-yr storm event to determine the effect of stormwater runoff from the proposed development to existing downstream drainage facilities outside the area of the subdivision. Impacts to the existing downstream drainage facilities shall be addressed in accordance with the requirements of the agency having jurisdiction.
- 5) Storm drainage pipes will be sized based on Manning's Equation (with n= 0.012 for RCP and HDPE sewer pipes).
 - a) Minimum pipe slope shall be 0.50% for pipes up to 15" diameter. Minimum pipe slope for pipes 18" diameter or larger shall follow the Town ordinance minimum of 0.05%.
 - b) Minimum storm sewer pipe shall be 15" diameter.
 - c) Maximum distance between inlet and manhole structures on roads will be 300'.
 - d) Each catch basin shall be designed in accordance with acceptable flow rates to the specific basin, but not to exceed 6 cfs in any case.
- b. Storm Drain Culvert
 - 1) The Soil Conservation Service Method TR20 shall be used for designing storm drain culverts.
 - Storm drainage culverts for roadways and pavements are to be designed for a 50 year storm, except that any facilities provided at a low point of the pavement/structure shall be able to pass a 100-year storm under surcharge conditions without flooding the roadway.
 - 3) For watersheds with drainage areas of less than 320 acres, the drainage culvers will be designed to convey the peak runoff for a 25-year storm event. For watersheds with drainage areas of between 320 and 640 acres, the drainage culverts will be designed to convey the peak runoff for a 50-year storm event. For watersheds with drainage areas larger than one square mile, the drainage culverts will be designed to convey the peak runoff for a 100-year storm event.
 - 4) The type of culvert structure material shall be selected based on size, structural strength, constructability, preservation of natural stream channel, aesthetics and cost.

5) In vehicular areas, provide a minimum cover of 18 inches or Class V RCP where cover is less than 18 inches.

2. Stormwater Detention Basins

- a. Hydrology for stormwater detention basins shall be based on USDA Soil Conservation Service Technical Release TR-20 methodology.
- b. Where detention facilities are required, the release of stormwater runoff to offsite areas shall not exceed the pre-development peak rate of runoff. To accomplish this, the rate of stormwater runoff shall be controlled through the use of detention basins or underground detention facilities, so that the post-development discharge rate is equal to or less than the existing discharge rate.
- c. The runoff generated from a 2 year, 10 year, and a 100 year storm shall be based on the 24 hour SCS Type III cumulative rainfall distribution for both existing and proposed conditions.
- d. Unless a site-specific soils evaluation is provided, the Rockland and Orange County soils maps shall be used to determine soil types to calculate CN factors. Additionally, the Town can require soil testing for specific locations.
- e. The water quantity basin design shall conform to the current design requirements contained in the <u>NYS Stormwater Management Design Manual</u>.

3. Water Quality Basin Design

- a. The water quality basin design shall conform to the current design requirements contained in the <u>NYS Stormwater Management Design Manual</u>.
- b. The design approach shall utilize a "kit of parts" philosophy that encourages a variety of stormwater management practice (SMP) types. The selection of a specific SMP shall be based upon guidance from Chapter 7 of the Design Manual, and shall be depend on topography, soil and groundwater conditions, habitat, watershed characteristics, aesthetics, and other pertinent factors. Examples of SMP types to be considered from Chapter 6 of the <u>NYS Stormwater Management Design Manual</u> include:
 - ponds
 - wetlands
 - infiltration practices (infiltration trenches, basins, dry wells)
 - filtering systems (sand filters, organic filters, bioretention facilities)
 - open channels (wet swales, dry swales).

Where practicable, alternative approaches from Chapter 9 of the <u>NYS Stormwater</u> <u>Management Design Manual</u> shall also be considered to mitigate potential adverse impacts. These measures include:

- rain gardens
- green roofs
- stormwater planters
- permeable paving
- cisterns.
- c. Where appropriate, water quality basins may be designed and incorporated as part of stormwater detention basin systems.

- d. Underground water quality treatment units may be used to satisfy the NYSDEC "pretreatment" criteria.
- e. Water quality treatment practices may include small decentralized facilities to serve localized drainage areas.

4. Open Channels

- a. Side slopes should not be greater than 2H:1V.
- b. Channels shall have a capacity to convey the runoff from a 25-year storm event with 0.5 foot of free board.
- c. The top width of parabolic waterway shall not exceed 30'; and the bottom width of trapezoidal waterway shall not exceed 15 feet.
- d. Channel stabilization measures shall be provided as required to prevent erosion.
- e. Rock Catchment areas may also be used to convey storm water runoff.

B. GRADING & EARTHWORK AND STEEP SLOPE PROTECTION

1. Grading & Earthwork

To minimize impacts to the natural resources, grading activities will be kept to the absolute minimum. Development activities will be generally limited to within those areas where the ground surface slope is no greater than 33%. The design of the roads and building areas will be completed to maintain less than a 20 foot cut or fill where ever possible. There will be those areas within the project site with existing topographic conditions which will require cuts and/or fills to be greater than 20 feet. Areas with cuts and/or fills greater than 20 feet will require evaluation and recommendations from the geotechnical engineer.

- a. Maximum slope for embankments and all landscaping areas shall be 2 horizontal to 1 vertical. Rock cut slopes and rock catchment areas will be based on the analysis and recommendations of the project geotechnical engineer.
- b. Grading plans to be designed to produce a minimum disturbance to natural resources. Balance earthwork where possible. Reuse excess rock for general fill and, if possible, for slope stabilization (i.e.; gabion walls) or for channel stabilization.
- c. Erosion control will be in compliance with New York State Guidelines. An erosion control plan will accompany all grading, utility and paving plans for both on-site and off-site improvements.
- d. Border areas shall be provided along Major, Collector, Local, Country and private roads. Guide rail should be considered where minimum border area cannot be provided.
 - (1) Major and Collector Road Border Area = min 9 ft wide measured from the edge of cartway. Maximum slope 3H:1V.
 - (2) Local, Country and Private Road Border Area = min 6 ft wide measured from the edge of cartway. Maximum slope 3H:1V.
- e. Retaining walls, rock catchments and slopes shall be permitted within the road R.O.W. provided these elements are located outside of the road border area or coordinated with guide rail protection.

2. Blasting

See Rock Blasting and Stabilization Protocol attached as Exhibit A.

C. ROAD STANDARDS

1. Streets

Street hierarchy and guidelines for gradients, sight distances, intersections, horizontal and vertical alignment, driveways, curbs and sidewalk have been addressed in the SMART Code.

2. Paving

Pavement sections will be based on Geotechnical Engineers recommendations and after completing a paving design analysis. The following are typical bituminous pavement sections to be used.

Major:	 1½" Asphalt Concrete Top Course, NYSDOT HMA 12.5 mm or 9.5 mm 1½" Asphalt Concrete Binder, NYSDOT HMA 25 mm 4" Asphalt Concrete Base Course, NYSDOT HMA 37.5 mm 8" Subbase, NYSDOT Type 1
Collector:	2" Asphalt Concrete Top Course, NYSDOT HMA 12.5 mm or 9.5 mm 4" Asphalt Concrete Base Course, NYSDOT HMA 37.5 mm 8" Subbase, NYSDOT Type 1
Residential, Local, Private: Country Lane	1½" Asphalt Concrete Top Course, NYSDOT HMA 12.5 mm or 9.5 mm 3" Asphalt Concrete Base Course, NYSDOT HMA 25 mm 8" Subbase, NYSDOT Type 1
Parking Lots:	1 ¹ / ₂ " Asphalt Concrete Top Course, NYSDOT HMA 12.5 mm or 9.5 mm 3" Asphalt Concrete Base Course, NYSDOT 6" Subbase, NYSDOT Type 1

D. SANITARY SEWER

Sanitary sewers shall be designed based on the standards and regulations of Orange County Health Departments and the NYS DEC Design Standards for Wastewater Treatment Works.

1. Wastewater Design Flow

The flow rate is based on the NYS DEC Design Standards for Wastewater Treatment Works latest edition, Table B-3. Except for the 110/130/150 gpd per unit values, the per-unit hydraulic loading rates in Table B-3 may be reduced by 20 percent for establishments equipped with water saving plumbing fixtures.

Assumed bank, postal office, food, sport/health, day care, retail, office & business center all based on the flow rate of 0.10 gpd/sf in Southern Tract; and 0.08 gpd/sf for office, light industrial and warehouse in Northern Tract.

Wastewater peak wet weather design flows will be based on the following:

- Domestic sanitary flows based on above average daily domestic sanitary flow rate times a peak hourly factor of 3.3.
- Infiltration flow allowance (included as part of the 3.3 peak hourly flow factor).

2. Gravity Sewer

- a. Pipe
 - Gravity system shall be designed for peak wet weather flow and without surcharging the system. Gravity sewers shall be designed when possible to provide a minimum velocity of not less than 2 fps when flowing half full. During initial years of development frequent sewer line flushing or cleaning may be required to flush and sediments that may be deposited during the low flows.
 - 2) Minimum diameter of sewer lateral shall be 6" for commercial connection, 4" for residential connection, and 8" for public sewer extension.
 - 3) Cleanout shall be provided at each lateral connection.
 - Sanitary pipes will be designed with following minimum hydraulic slope for all pipe types using n=0.013 for ductile iron and n=0.010 for PVC to provide a minimum velocity of 2 fps.
 - 5) The maximum velocity shall be 15 fps, where 10 fps is preferred where possible.
 - 6) When a smaller sewer joins a larger one, the invert of the larger sewer should be lowered to place the 0.8 depth point of both sewers at the same elevation.
 - 7) Minimum cover above sanitary sewer pipe shall be 4'.
 - 8) Minimum 3' of cover from the bottom of stream at stream crossing.
 - 9) Minimum 10' of lateral separation and 18" of vertical separation shall be provided between sanitary sewers and water main.
 - 10) Where appropriate separation from a water main is not possible, the sewer shall be encased in concrete or constructed of ductile iron pipe using mechanical or slip-on joints for a distance of at least 10' on either side of the crossing.
 - 11) No sewer main shall be constructed within:
 - 200' from a water supply well or a below-grade reservoir, unless otherwise approved by the Orange County Department of Health;
 - 25' from surface water or open drainage/ culvert;
 - 10' from a water pipe;
 - 25' from top of embankment or steep slope.
 - 12) Sanitary sewers crossing streams or wetlands, or located within 25' of the stream embankment shall be constructed of steel, reinforced concrete or ductile iron pipes.
 - 13) Maximum distance between manholes shall not exceed 400 feet.
 - 14) Sewers on 20% slopes or greater shall be anchored securely with concrete or equal, anchors spaced as below:
 - Not over 36' center to center on grades 20% and up to 35%;
 - Not over 24' center to center on grades 35% and up to 50%;
 - Not over 16' center to center on grades 50% and over.

- 15) Except where otherwise approved by municipality or utility authority, the centerline of sanitary sewer manholes, when located within the municipal right-of-way, shall be located where possible 5ft from the center line of the paved cartway. If conditions prevent location near the centerline, the manhole shall be located within the cartway a minimum of five-feet from the gutterline.
- b. Sanitary Sewer Manholes
 - Minimum inside manhole diameter for a standard sanitary sewer manhole shall be 4848". All other manholes will comply with NYS DEC Design Standards for Wastewater Treatment Works latest edition, Table C-1.
 - 2) Provide minimum 0.1' drop across the sanitary sewer manhole.
 - Provide a drop connection when the difference between inverts is more than 2 feet. Inside or outside drop shall be used. Inside drop connections shall require a larger inside diameter manhole
 - 4) No manholes are permitted within 100' of a public water supply well or a below grade reservoir.
 - 5) Watertight manhole cover shall be provided within the 100-year flood level or 50' from the wetlands limit.

3. Force Main

- a. The minimum building service connections from individual grinder pumps to the collectors shall be 1-1/4" PVC pipe.
- b. The minimum force main diameter shall be 4".
- c. The minimum velocity shall be 2 fps.
- d. Force main shall enter the gravity sewer system at a point not more than 2 feet above the flow line of the receiving manhole.
- e. Cleanout shall be provided every 400' to 500', at major change in direction, and at where one collector main joins another main.

4. Pump Station

- a. Pump station shall be located outside of the 100-year floodplain and be accessible during the 25 year flood.
- b. Pump suction and discharge openings shall be at least 4" in diameter, unless otherwise approved in writing by the Town.

5. Sewage Disposal System

a. All design and construction of sewage disposal systems shall be in accordance with the standards and regulations of Orange County Department of Health, and New York Department of Environmental Conservation, as outlined in the with Appendix 75-A of Part 75 of the Administrative Rules and Regulations contained in Chapter 11 of Title 10 (Health) of the Official Compilation of Codes, rules and Regulations of the State of New York.

b. Mounded septic system is not permitted in Orange County

E. WATER

Water mains, pumping facilities and distribution storage design shall be designed in accordance with the latest edition of Recommended Standards for Water Work (also known as the Ten State Standards) and Part 5 of the New York State Sanitary Sewer Code.

1. Water Demand

Water demand is the sum of the residential and non-residential water demand. Fire flow volume is provided in dedicated storage and is, therefore, not a demand. The residential and non-residential water demand will be based on the following:

Parameter	Water per Unit Basis	2022 Development Program	Water Demand		
Residential					
1BR Dwellings	110 gal/unit/day	285 units	31,350 gpd		
2BR Dwellings	220 gal/unit/day	858 units	188,760 gpd		
3BR Dwellings	330 gal/unit/day	351 units	115,830 gpd		
4BR Dwellings	440 gal/unit/day	115 units	50,600 gpd		
Residential Average Daily Demand			386,540 gpd		
Non-Residential					
Commercial Non- Amenity Uses	0.10 gal/sf/day	42,000 sq. feet	4,200 gpd		
Amenity Uses (used by development residents, water / wastewater demands included in bedroom count)	0. 0 gal/sf/day	61,000 sq. feet	0 gpd		
Non-Residential Average Daily Demand			4,200 gpd		
Totals					
Total Avg Daily Demand			390,740 gpd		
Total Maximum Day Demand		1.76	687,700 gpd		
Total Available Max Day Supply			835,200 gpd		
(24 hrs per day)					
Notes:					
1) Up to 3,620 total bedrooms, with a maximum of 3,070 non-age restricted bedrooms					
 Weighted Average Peaking Factor is calculated as follows: [(SFD X 2.5) + (MFD X 1.5)] / Total Units (SFD) Single Family Dwellings: 413 units (MFD) Multi Family Dwellings: 1,196 units Total: 1,609 units Peaking Factor: [(413 X 2.5) + (1,196 X 1.5)] / 1,609 = 1.76 					

The water supply system should be designed to carry and deliver the peak-hour flow demand and the fire flow requirement whichever is greater. The storage facility shall contain average day demand plus the fire flow requirement.

2. Fire Flow

The fire protection need will be based on Insurance Services Office (ISO), *Fire Suppression Rating Schedule* and the ISO formula for estimated Needed Fire Flow (NFF) and the American Water Works Association's (AWWA). Manual of Practice 31, *Distribution System Requirements for Fire Protection* (M31).

The following fire flow values will be provided:

- 2000 gpm for 2 hours for the multi-family dwellings and townhomes
- 1060 gpm for 2 hours will be provided to the school
- 1000 gpm for 2 hours will be provided for the single family homes.

The minimum fire flow requirement is based on 2000 gpm for two hours.

3. Fire Hydrant

- a. Fire hydrants shall be spaced in accordance with New York State requirements.
- b. Fire hydrants shall be generally located in the vicinity of low and high points of the streets and in the vicinity of street dead ends.

4. Water Main

- a. All water mains shall be sized to maintain a minimum pressure of 20 psi at ground level at all points in the distribution system under all conditions of flow, including fire.
- b. The normal working pressure in the distribution system should be approximately 60 psi and not less than 35 psi.
- c. Water main shall be separated both horizontally by 10' minimum and vertically by 18" minimum with any pipe lines carrying non-potable water such as sanitary and storm sewer.
- d. Water mains will be installed with minimum 4 feet of cover.
- e. Minimum water main size shall be 6" diameter CC-900 PVC or as approved by the Orange County Department of Health.
- f. Maximum valve spacing shall be 500' for commercial and 800' for residential development.
- g. Water mains shall be installed with minimum 2 feet of cover when crossing water course 15 feet wide. The pipe shall be of special construction having flexible watertight joint, and valves and permanent taps shall be provided on both sides of major stream crossings.

5. Water Storage Tank

- a. Water Storage tank shall be tucked below the ridge lines where possible.
- b. The minimum capacity of a water storage facility shall contain average day demand and fire flow. The tank has 672,000 gallons of storage and an associated average day capacity of 432,000 gpd when accounting for a 2 hour fire flow of 2,000 gpm.

6. Wells

Public water supply wells shall be located to meet State regulatory requirements.

F. SOIL EROSION AND SEDIMENT CONTROL

Soil Erosion and sediment control devices and application shall be in accordance with New York State Standards and Specifications for Erosion and Sediment Control.

G. TREE SURVEYS

Given the intention of clustered development at Tuxedo Reserve is to preserve large swaths of open space, it is understood that construction activities within the limits of disturbance of roads and lots will generally require the clear-cutting of trees. In addition to clearing, grading activities will generally prohibit the ability to preserve specific trees on proposed lots. To mitigate the visual impact of these clearing and grading activities, the Special Permit adopted the Design Guidelines which includes standards for on-lot landscaping.

The Planning Board may require a tree survey and protective measures to ensure the survival of the tree or stand of trees in the specific circumstances when a specimen tree or a stand of specimen trees (pertaining to trees 18 inches caliper DBH and larger) are located within the limits of disturbance and:

- 1. The preservation of the tree or trees does not significantly alter the alignment of a road or its associated infrastructure;
- 2. The preservation of the tree or trees does not eliminate or require the relocation of any lot or lots such that the new lot or lots have less usable land area;
- 3. The preservation of the tree or trees does not render any lot unbuildable, or substantially increase the cost or difficulty of developing a lot;
- 4. The proposed grading is such that the tree's or stand of trees' survival is feasible.

H. WATER QUALITY TESTING

To ensure the protection and preservation of the water quality at Tuxedo Lake, Mountain Lake, and on the site, in general, the applicant must test surface water samples following any storm greater than 1.5 inches, or the water quality storm rainfall provided in the New York State Stormwater Management Design Manual, latest edition whichever is more restrictive, as follows:

Construction Occurring Within:	Test samples taken from:	Any time before construction, test for:	During construction, test for:	Six months post- construction and soil stabilization, test for:
All Phases	The existing stream on the Sloatsburg parcel before it enters the culvert in Route 17	 Total suspended solids (TSS) Total phosphorus Total nitrogen pH Chloride 	 Total suspended solids (TSS) If a spill occurs, test for appropriate 	 Total suspended solids (TSS) Total phosphorus Total nitrogen
1,000 feet of the nearest edge of Mountain Lake	The existing swale near the proposed recreation facility		substances (e.g. petroleum products)	pHChloride
The Tuxedo Lake watershed	The outlets from sediment traps at the locations of Pocket Pond 5a and Dry Swale 5b*			

*Note: Only test outlet from sediment trap at the location of Pocket Pond 5a for pre-construction test.

I. ENVIRONMENTAL COMPLIANCE

Prior to the first site disturbance, the developer must submit an Environmental Compliance Document to the Town Engineer for his review and approval. It is the responsibility of the developer to maintain signed copies of the Environmental Compliance Document for each of the Project's contractors at the Project's construction office. In addition, copies of the executed Environmental Compliance Document must be attached to each Clearing and Grading Permit and Building Permit.

The approved Environmental Compliance Document must require compliance with the Project's NYSDEC-approved Stormwater Pollution Prevention Plans as well as identify additional measures for the following:

- Spill Protection: Preventative measures, emergency spill event action plan, and nonemergency spill event action plan;
- Hazardous Waste Management: Identification, reporting, storage, and disposal;
- Solid Waste Management: Storage and disposal; and
- Natural & Cultural Resources: Identification and reporting.